LOADS

High performance anchor FH II - SK

Highest permissible loads for a single anchor¹⁾ in concrete C20/25⁴⁾

For the design the complete approval ETA - 07/0025 has to be considered.

				Cracked concrete				Non-cracked concrete			
Туре	Effective anchorage depth	Min. member thickness		Permissible tensile load		Min. spacing	Min. edge distance	Permissible tensile load	Permissible shear load	Min. spacing	Min. edge distance
	h _{ef}	h _{min}	T _{inst}	N _{perm} 3)	V _{perm} 3)	s _{min} 2)	c _{min²⁾}	N _{perm} 3)	V _{perm} 3)	s _{min} ²⁾	c _{min²⁾}
	[mm]	[mm]	[Nm]	[kN]	[kN]	[mm]	[mm]	[kN]	[kN]	[mm]	[mm]
FH II 10 SK	40	80	10,0	3,6	4,3	40	40	6,1	6,1	40	40
FH II 12 SK	60	120	22,5	5,7	15,9	50	50	11,2	18,9	60	60
FH II 15 SK	70	140	40,0	7,6	20,1	60	60	14,1	28,2	70	70
FH II 18 SK	80	160	80,0	11,9	24,5	70	70	17,2	34,4	80	80
The partial safety factors for material resistance as regulated in the approval as well as a partial 3 For combinations of tensile loads, shear loads, bending moments as well as reduced edge											d edge

¹⁾ The partial safety factors for material resistance as regulated in the approval as well as a partial safety factor for load actions of γ_L = 1,4 are considered. As an single anchor counts e.g. an anchor with a spacing s \geq 3 x h_{ef} and an edge distance c \geq 1,5 x h_{ef}. Accurate data see approval.

²⁾ Minimum possible axial spacings resp. edge distance while reducing the permissible load.

distances or spacings (anchor groups) see approval.

 $^{^{\}rm 4)}$ For higher concrete strength classes up to C50/60 higher permissible loads may be possible.

LOADS

High performance anchor FH II - SK A4

Highest permissible loads for a single anchor¹⁾ in concrete C20/25⁴⁾

For the design the complete approval ETA - 07/0025 has to be considered. Cracked concrete Non-cracked concrete Type **Effective** Min. Installation permissible permissible min. permissible permissible min. min. min. tensile tensile anchorage member torque shear spacing edae shear spacing edge thickness distance depth load load load load distance N_{perm}3) V_{perm}3) Smin²⁾ c_{min²⁾} N_{perm}3) V_{perm}3) Smin²⁾ C_{min²⁾} hef h_{min} Tinst [kN] [kN] [kN] [mm] [mm] [Nm] [kN] [mm] [mm] [mm] [mm] **FH II 12 SK A4** 60 120 25.0 5.7 15.9 50 50 9.5 16.0 60 60 **FH II 15 SK A4** 70 140 40.0 7.6 20.1 60 60 14.1 24.6 70 70 **FH II 18 SK A4** 80 160 100.0 11.9 24.5 70 70 17.2 34.4 80 80

The partial safety factors for material resistance as regulated in the approval as well as a partial 3) For combinations of tensile loads, shear loads, bending moments as well as reduced edge safety factor for load actions of $\gamma_1 = 1.4$ are considered. As an single anchor counts e.g. an anchor with a spacing $s \ge 3$ x h_{ef} and an edge distance $c \ge 1.5$ x h_{ef} . Accurate data see approval.

distances or spacings (anchor groups) see approval.

⁴⁾ For higher concrete strength classes up to C50/60 higher permissible loads may be possible.

Minimum possible axial spacings resp. edge distance while reducing the permissible load.