

LOADS

High performance anchor FH II - S

Highest permissible loads for a single anchor¹⁾ in concrete C20/25⁴⁾

For the design the complete approval ETA - 07/0025 has to be considered.

Type	Effective anchorage depth h_{ef} [mm]	Min. member thickness h_{min} [mm]	Installation torque T_{inst} [Nm]	Cracked concrete				Non-cracked concrete			
				Permissible tensile load	Permissible shear load	Min. spacing	Min. edge distance	Permissible tensile load	Permissible shear load	Min. spacing	Min. edge distance
				$N_{perm}^{3)}$ [kN]	$V_{perm}^{3)}$ [kN]	$s_{min}^{2)}$ [mm]	$c_{min}^{2)}$ [mm]	$N_{perm}^{3)}$ [kN]	$V_{perm}^{3)}$ [kN]	$s_{min}^{2)}$ [mm]	$c_{min}^{2)}$ [mm]
FH II 10 S	40	80	10,0	3,6	4,3	40	40	6,1	6,1	40	40
FH II 12 S	60	120	22,5	5,7	15,9	50	50	11,2	18,9	60	60
FH II 15 S	70	140	40,0	7,6	20,1	60	60	14,1	28,2	70	70
FH II 18 S	80	160	80,0	11,9	24,5	70	70	17,2	34,4	80	80
FH II 24 S	100	200	160,0	17,1	34,3	80	80	24,0	48,1	100	100
FH II 28 S	125	250	180,0	24,0	47,9	100	100	33,6	67,2	120	120
FH II 32 S	150	300	200,0	31,5	63,0	120	120	44,2	88,4	160	180

¹⁾ The partial safety factors for material resistance as regulated in the approval as well as a partial safety factor for load actions of $\gamma_L = 1,4$ are considered. As a single anchor counts e.g. an anchor with a spacing $s \geq 3 \times h_{ef}$ and an edge distance $c \geq 1,5 \times h_{ef}$. Accurate data see approval.

²⁾ Minimum possible axial spacings resp. edge distance while reducing the permissible load.

³⁾ For combinations of tensile loads, shear loads, bending moments as well as reduced edge distances or spacings (anchor groups) see approval.

⁴⁾ For higher concrete strength classes up to C50/60 higher permissible loads may be possible.

LOADS

High performance anchor FH II - S A4

Highest permissible loads for a single anchor¹⁾ in concrete C20/25⁴⁾

For the design the complete approval ETA - 07/0025 has to be considered.

Type	Effective anchorage depth h_{ef} [mm]	Min. member thickness h_{min} [mm]	Installation torque T_{inst} [Nm]	Cracked concrete				Non-cracked concrete			
				permissible tensile load $N_{perm}^{3)}$ [kN]	permissible shear load $V_{perm}^{3)}$ [kN]	min. spacing $s_{min}^{2)}$ [mm]	min. edge distance $c_{min}^{2)}$ [mm]	permissible tensile load $N_{perm}^{3)}$ [kN]	permissible shear load $V_{perm}^{3)}$ [kN]	min. spacing $s_{min}^{2)}$ [mm]	min. edge distance $c_{min}^{2)}$ [mm]
FH II 10 S A4	40	80	15,0	3,6	4,3	40	40	6,1	6,1	40	40
FH II 12 S A4	60	120	25,0	5,7	15,9	50	50	9,5	16,0	60	60
FH II 15 S A4	70	140	40,0	7,6	20,1	60	60	14,1	24,6	70	70
FH II 18 S A4	80	160	100,0	11,9	24,5	70	70	17,2	34,4	80	80
FH II 24 S A4	100	200	160,0	17,1	34,3	80	80	24,0	48,1	100	100

¹⁾ The partial safety factors for material resistance as regulated in the approval as well as a partial safety factor for load actions of $\gamma_L = 1,4$ are considered. As an single anchor counts e.g. an anchor with a spacing $s \geq 3 \times h_{ef}$ and an edge distance $c \geq 1,5 \times h_{ef}$. Accurate data see approval.

²⁾ Minimum possible axial spacings resp. edge distance while reducing the permissible load.

³⁾ For combinations of tensile loads, shear loads, bending moments as well as reduced edge distances or spacings (anchor groups) see approval.

⁴⁾ For higher concrete strength classes up to C50/60 higher permissible loads may be possible.