



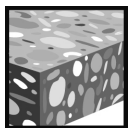


## Hilti HIT-HY 200 with HIT-V

Injection mortar system		Benefits
	<p>Hilti HIT-HY 200-A 500 ml foil pack (also available as 330 ml foil pack)</p>	<ul style="list-style-type: none"> <li>- Suitable for non-cracked and cracked concrete C 20/25 to C 50/60</li> <li>- Suitable for dry and water saturated concrete</li> <li>- High loading capacity, excellent handling and fast curing</li> <li>- Small edge distance and anchor spacing possible</li> <li>- Large diameter applications</li> <li>- Max In service temperature range up to 120°C short term/ 72°C long term</li> <li>- Manual cleaning for borehole diameter up to 20mm and <math>h_{ef} \leq 10d</math> for non-cracked concrete only</li> <li>- Embedment depth range: from 60 ... 160 mm for M8 to 120 ... 600 mm for M30</li> <li>- Two mortar (A and R) versions available with different curing times and same performance</li> </ul>
	<p>Hilti HIT-HY 200-R 500 ml foil pack (also available as 330 ml foil pack)</p>	
	<p>Static mixer</p>	
	<p>HIT-V rods HIT-V-R rods HIT-V-HCR rods</p>	



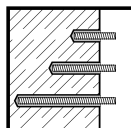
Concrete



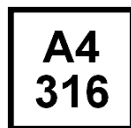
Tensile zone



Small edge distance and spacing



Variable embedment depth



Corrosion resistance



High corrosion resistance



European Technical Approval



CE conformity



PROFIS Anchor design software

### Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval <sup>a)</sup>	DIBt, Berlin	ETA-11/0493 / 2012-08-08 (Hilti HIT-HY 200-A) ETA-12/0084 / 2012-08-08 (Hilti HIT-HY 200-R)
Fire test report	IBMB, Brunswick	3501/676/13 / 2012-08-03

a) All data given in this section according ETA-11/0493 and ETA-12/0084, issue 2012-08-08.

## Basic loading data (for a single anchor)

All data in this section applies to

For details see Simplified design method

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- One typical embedment depth, as specified in the table
- One anchor material, as specified in the tables
- Concrete C 20/25,  $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I  
(min. base material temperature  $-40^\circ\text{C}$ , max. long term/short term base material temperature:  $+24^\circ\text{C}/40^\circ\text{C}$ )
- Installation temperature range  $-10^\circ\text{C}$  to  $+40^\circ\text{C}$

### Embedment depth <sup>a)</sup> and base material thickness for the basic loading data. Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Typical embedment depth $h_{ef}$ [mm]	80	90	110	125	170	210	240	270
Base material thickness $h$ [mm]	110	120	140	165	220	270	300	340

a) The allowed range of embedment depth is shown in the setting details. The corresponding load values can be calculated according to the simplified design method.

### Mean ultimate resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile $N_{Ru,m}$ HIT-V 5.8 [kN]	18,9	30,5	44,1	83,0	129,2	185,9	241,5	295,1
Shear $V_{Ru,m}$ HIT-V 5.8 [kN]	9,5	15,8	22,1	41,0	64,1	92,4	120,8	147,0
Cracked concrete								
Tensile $N_{Ru,m}$ HIT-V 5.8 [kN]	16,0	22,5	44,0	66,7	105,9	145,4	177,7	212,0
Shear $V_{Ru,m}$ HIT-V 5.8 [kN]	9,5	15,8	22,1	41,0	64,1	92,4	120,8	147,0

### Characteristic resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile $N_{Rk}$ HIT-V 5.8 [kN]	18,0	29,0	42,0	70,6	111,9	153,7	187,8	224,0
Shear $V_{Rk}$ HIT-V 5.8 [kN]	9,0	15,0	21,0	39,0	61,0	88,0	115,0	140,0
Cracked concrete								
Tensile $N_{Rk}$ HIT-V 5.8 [kN]	12,1	17,0	33,2	50,3	79,8	109,6	133,9	159,7
Shear $V_{Rk}$ HIT-V 5.8 [kN]	9,0	15,0	21,0	39,0	61,0	88,0	115,0	140,0

### Design resistance: concrete C 20/25 , anchor HIT-V 5.8

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete								
Tensile $N_{Rd}$ HIT-V 5.8 [kN]	12,0	19,3	28,0	39,2	62,2	85,4	104,3	124,5
Shear $V_{Rd}$ HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
Cracked concrete								
Tensile $N_{Rd}$ HIT-V 5.8 [kN]	6,7	9,4	18,4	27,9	44,3	60,9	74,4	88,7
Shear $V_{Rd}$ HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0

### Recommended loads <sup>a)</sup>: concrete C 20/25 , anchor HIT-V 5.8

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete										
Tensile $N_{rec}$	HIT-V 5.8	[kN]	8,6	13,8	20,0	28,0	44,4	61,0	74,5	88,9
Shear $V_{rec}$	HIT-V 5.8	[kN]	5,1	8,6	12,0	22,3	34,9	50,3	65,7	80,0
Cracked concrete										
Tensile $N_{rec}$	HIT-V 5.8	[kN]	4,8	6,7	13,2	19,9	31,7	43,5	53,1	63,4
Shear $V_{rec}$	HIT-V 5.8	[kN]	5,1	8,6	12,0	22,3	34,9	50,3	65,7	80,0

a) With overall partial safety factor for action  $\gamma = 1,4$ . The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

### Service temperature range

Hilti HIT-HY 200 injection mortar may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

#### Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

#### Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

## Materials

### Mechanical properties of HIT-V

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Nominal tensile strength $f_{uk}$	HIT-V 5.8	[N/mm <sup>2</sup> ]	500	500	500	500	500	500	500	500
	HIT-V 8.8	[N/mm <sup>2</sup> ]	800	800	800	800	800	800	800	800
	HIT-V-R	[N/mm <sup>2</sup> ]	700	700	700	700	700	700	500	500
	HIT-V-HCR	[N/mm <sup>2</sup> ]	800	800	800	800	800	700	700	700
Yield strength $f_{yk}$	HIT-V 5.8	[N/mm <sup>2</sup> ]	400	400	400	400	400	400	400	400
	HIT-V 8.8	[N/mm <sup>2</sup> ]	640	640	640	640	640	640	640	640
	HIT-V -R	[N/mm <sup>2</sup> ]	450	450	450	450	450	450	210	210
	HIT-V-HCR	[N/mm <sup>2</sup> ]	640	640	640	640	640	400	400	400
Stressed cross-section $A_s$	HIT-V	[mm <sup>2</sup> ]	36,6	58,0	84,3	157	245	353	459	561
Moment of resistance $W$	HIT-V	[mm <sup>3</sup> ]	31,2	62,3	109	277	541	935	1387	1874

### Material quality

Part	Material
Threaded rod HIT-V(F)	Strength class 5.8, A <sub>5</sub> > 8% ductile steel galvanized ≥ 5 μm, (F) hot dipped galvanized ≥ 45 μm,
Threaded rod HIT-V(F)	Strength class 8.8, A <sub>5</sub> > 8% ductile steel galvanized ≥ 5 μm, (F) hot dipped galvanized ≥ 45 μm,
Threaded rod HIT-V-R	Stainless steel grade A4, A <sub>5</sub> > 8% ductile strength class 70 for ≤ M24 and class 50 for M27 to M30, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
Threaded rod HIT-V-HCR	High corrosion resistant steel, 1.4529; 1.4565 strength ≤ M20: R <sub>m</sub> = 800 N/mm <sup>2</sup> , R <sub>p0.2</sub> = 640 N/mm <sup>2</sup> , A <sub>5</sub> > 8% ductile M24 to M30: R <sub>m</sub> = 700 N/mm <sup>2</sup> , R <sub>p0.2</sub> = 400 N/mm <sup>2</sup> , A <sub>5</sub> > 8% ductile
Washer ISO 7089	Steel galvanized, hot dipped galvanized,
	Stainless steel, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
	High corrosion resistant steel, 1.4529; 1.4565
Nut EN ISO 4032	Strength class 8, steel galvanized ≥ 5 μm, hot dipped galvanized ≥ 45 μm,
	Strength class 70, stainless steel grade A4, 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362
	Strength class 70, high corrosion resistant steel, 1.4529; 1.4565

### Anchor dimensions

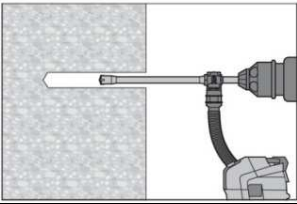
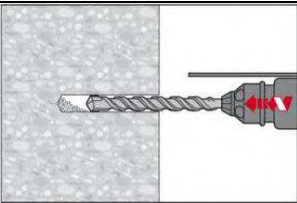
Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Anchor rod HIT-V, HIT-V-R, HIT-V-HCR	Anchor rods HIT-V (-R / -HCR) are available in variable length							

### Setting

#### installation equipment

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Rotary hammer	TE 2 – TE 16				TE 40 – TE 70			
Other tools, hammer drilling	compressed air gun or blow out pump, set of cleaning brushes, dispenser							

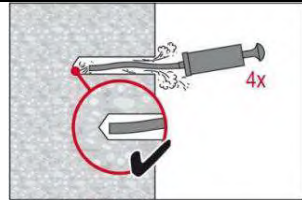
#### Setting instruction

Bore hole drilling	
	Drill hole to the required embedment depth with an appropriately sized Hilti TE-CD or TE-YD hollow drill bit with Hilti vacuum attachment. This drilling method properly cleans the borehole and removes dust while drilling. After drilling is complete, proceed to the "injection preparation" step in the instructions for use.
	Drill Hole to the required embedment depth with a hammer drill set in rotation-hammer mode using an appropriately sized carbide drill bit.

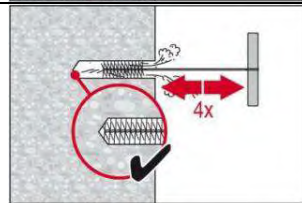
**Bore hole cleaning** Just before setting an anchor, the bore hole must be free of dust and debris.

**a) Manual Cleaning (MC) non-cracked concrete only**

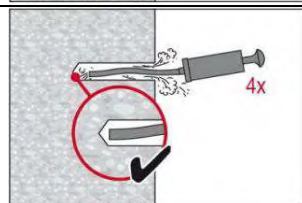
for bore hole diameters  $d_0 \leq 20\text{mm}$  and bore hole depth  $h_0 \leq 10d$



The Hilti manual pump may be used for blowing out bore holes up to diameters  $d_0 \leq 20\text{ mm}$  and embedment depths up to  $h_{ef} \leq 10d$ . Blow out at least 4 times from the back of the bore hole until return air stream is free of noticeable dust



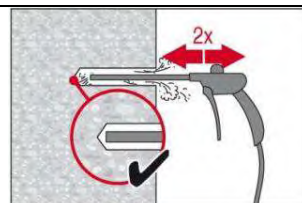
Brush 4 times with the specified brush size by inserting the steel brush Hilti HIT-RB to the back of the hole (if needed with extension) in a twisting motion and removing it. The brush must produce natural resistance as it enters the bore hole -- if not the brush is too small and must be replaced with the proper brush diameter.



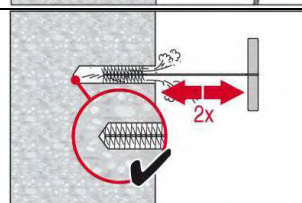
Blow out again with manual pump at least 4 times until return air stream is free of noticeable dust.

**b) Compressed air cleaning (CAC)**

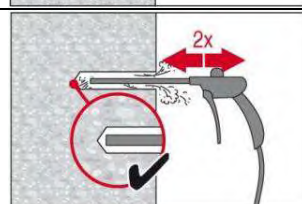
for all bore hole diameters  $d_0$  and all bore hole depth  $h_0$



Blow 2 times from the back of the hole (if needed with nozzle extension) over the hole length with oil-free compressed air (min. 6 bar at 6 m<sup>3</sup>/h) until return air stream is free of noticeable dust. Bore hole diameter  $\geq 32\text{ mm}$  the compressor must supply a minimum air flow of 140 m<sup>3</sup>/hour.

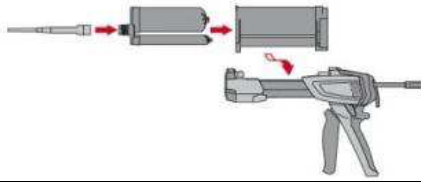


Brush 2 times with the specified brush size by inserting the steel brush Hilti HIT-RB to the back of the hole (if needed with extension) in a twisting motion and removing it. The brush must produce natural resistance as it enters the bore hole -- if not the brush is too small and must be replaced with the proper brush diameter.

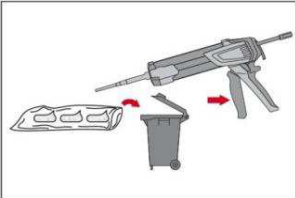


Blow again with compressed air 2 times until return air stream is free of noticeable dust.

### Injection preparation



Tightly attach new Hilti mixing nozzle HIT-RE-M to foil pack manifold (snug fit). Do not modify the mixing nozzle. Observe the instruction for use of the dispenser. Check foil pack holder for proper function. Do not use damaged foil packs / holders. Swing foil pack holder with foil pack into HIT-dispenser.

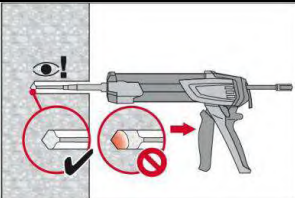


Discard initial adhesive. The foil pack opens automatically as dispensing is initiated. Depending on the size of the foil pack an initial amount of adhesive has to be discarded.

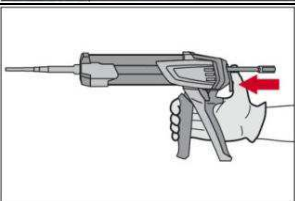
Discard quantities are:

- 2 strokes for 330 ml foil pack,
- 3 strokes for 500 ml foil pack,
- 4 strokes for 500 ml foil pack  $\leq 5^{\circ}\text{C}$ .

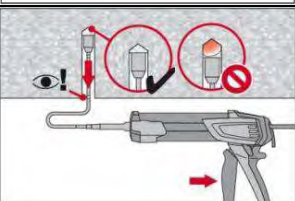
### Inject adhesive from the back of the borehole without forming air voids



Inject the adhesive starting at the back of the hole, slowly withdrawing the mixer with each trigger pull. Fill holes approximately 2/3 full, or as required to ensure that the annular gap between the anchor and the concrete is completely filled with adhesive along the embedment length.

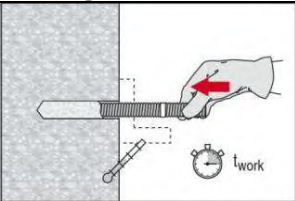


After injection is completed, depressurize the dispenser by pressing the release trigger. This will prevent further adhesive discharge from the mixer.

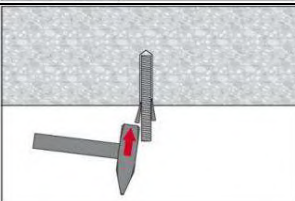


Overhead installation and/or installation with embedment depth  $h_{ef} > 250\text{mm}$ . For overhead installation the injection is only possible with the aid of extensions and piston plugs. Assemble HIT-RE-M mixer, extension(s) and appropriately sized piston plug. Insert piston plug to back of the hole and inject adhesive. During injection the piston plug will be naturally extruded out of the bore hole by the adhesive pressure.

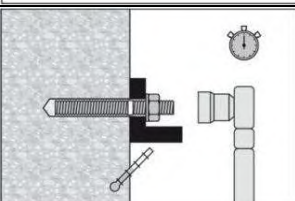
### Setting the element



Before use, verify that the element is dry and free of oil and other contaminants. Mark and set element to the required embedment depth until working time  $t_{work}$  has elapsed.



For overhead installation use piston plugs and fix embedded parts with e.g. wedges



Loading the anchor:  
After required curing time  $t_{cure}$  the anchor can be loaded.  
The applied installation torque shall not exceed  $T_{max}$ .

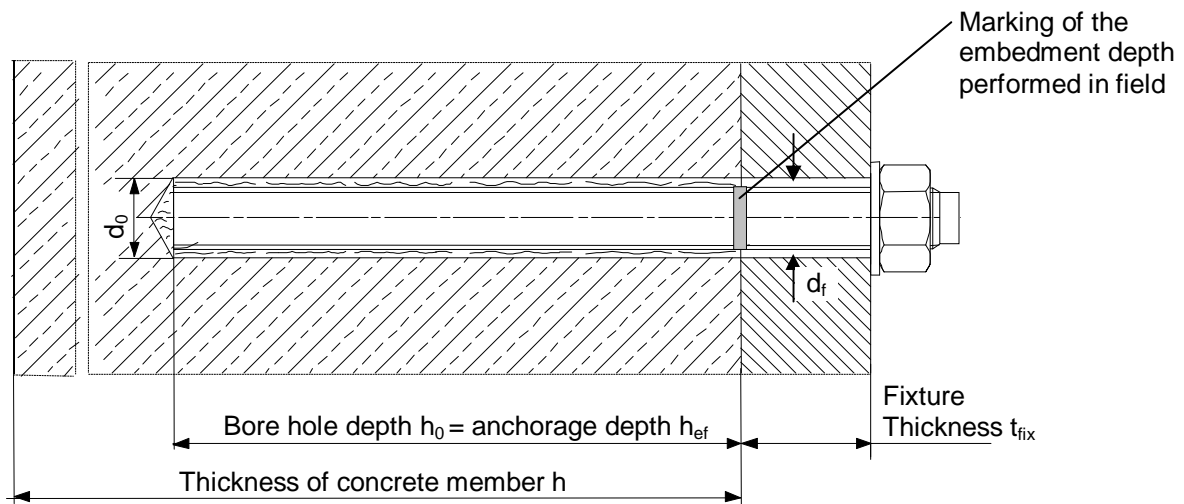
For detailed information on installation see instruction for use given with the package of the product.

### Working time, curing time

Temperature of the base material	Hilti HIT-HY 200-R	
	Working time in which anchor can be inserted and adjusted $t_{work}$	Curing time before anchor can be loaded $t_{cure}$
-10 °C to -5 °C	3 hour	20 hour
-4 °C to 0 °C	2 hour	7 hour
1 °C to 5 °C	1 hour	3 hour
6 °C to 10 °C	40 min	2 hour
11 °C to 20 °C	15 min	1 hour
21 °C to 30 °C	9 min	1 hour
31 °C to 40 °C	6 min	1 hour

Temperature of the base material	Hilti HIT-HY 200-A	
	Working time in which anchor can be inserted and adjusted $t_{work}$	Curing time before anchor can be loaded $t_{cure}$
-10 °C to -5 °C	1,5 hour	7 hour
-4 °C to 0 °C	50 min	4 hour
1 °C to 5 °C	25 min	2 hour
6 °C to 10 °C	15 min	1 hour
11 °C to 20 °C	7 min	30 min
21 °C to 30 °C	4 min	30 min
31 °C to 40 °C	3 min	30 min

### Setting details



### Setting details

Anchor size			M8	M10	M12	M16	M20	M24	M27	M30
Nominal diameter of drill bit	$d_0$	[mm]	10	12	14	18	22	28	30	35
Effective embedment and drill hole depth range <sup>a)</sup> <b>for HIT-V</b>	$h_{ef,min}$	[mm]	60	60	70	80	90	96	108	120
	$h_{ef,max}$	[mm]	160	200	240	320	400	480	540	600
Minimum base material thickness	$h_{min}$	[mm]	$h_{ef} + 30 \text{ mm}$			$h_{ef} + 2 d_0$				
Diameter of clearance hole in the fixture	$d_f$	[mm]	9	12	14	18	22	26	30	33
Torque moment	$T_{max}$ <sup>b)</sup>	[Nm]	10	20	40	80	150	200	270	300
Minimum spacing	$s_{min}$	[mm]	40	50	60	80	100	120	135	150
Minimum edge distance	$c_{min}$	[mm]	40	50	60	80	100	120	135	150
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$							
Critical edge distance for splitting failure <sup>c)</sup>	$c_{cr,sp}$	[mm]	<b>1,0 · h<sub>ef</sub></b> for $h / h_{ef} \geq 2,0$							
			<b>4,6 h<sub>ef</sub> - 1,8 h</b> for $2,0 > h / h_{ef} > 1,3$							
			<b>2,26 h<sub>ef</sub></b> for $h / h_{ef} \leq 1,3$							
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	$2 c_{cr,N}$							
Critical edge distance for concrete cone failure <sup>d)</sup>	$c_{cr,N}$	[mm]	$1,5 h_{ef}$							

For spacing (or edge distance) smaller than critical spacing (or critical edge distance) the design loads have to be reduced.

- a) Embedment depth range:  $h_{ef,min} \leq h_{ef} \leq h_{ef,max}$
- b) Maximum recommended torque moment to avoid splitting failure during installation with minimum spacing and/or edge distance.
- c)  $h$ : base material thickness ( $h \geq h_{min}$ ),  $h_{ef}$ : embedment depth
- d) The critical edge distance for concrete cone failure depends on the embedment depth  $h_{ef}$  and the design bond resistance. The simplified formula given in this table is on the safe side.



## Simplified design method

Simplified version of the design method according ETAG 001, TR 029. Design resistance according data given in ETA-11/0493 issued 2012-08-08 for HIT-HY 200-A and ETA-12/0084 issued 2012-08-08 for HIT-HY 200-R. Both mortars possess identical technical load performance.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The simplified calculated design loads take a conservative approach: They will be lower than the exact values according to ETAG 001, TR 029. For an optimized design, anchor calculation can be performed using PROFIS anchor design software.

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

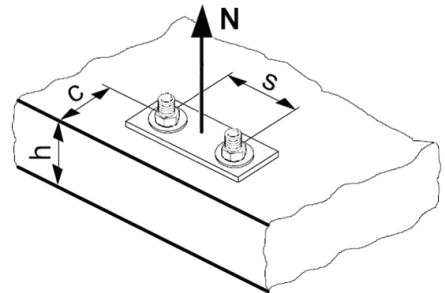
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

## TENSION loading

### The design tensile resistance is the lower value of

- Steel resistance:  $N_{Rd,s}$
- Combined pull-out and concrete cone resistance:  $N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$
- Concrete cone resistance:  $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete):  $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$



## Basic design tensile resistance

### Design steel resistance $N_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
$N_{Rd,s}$	HIT-V 5.8 [kN]	12,0	19,3	28,0	52,7	82,0	118,0	153,3	187,3
	HIT-V 8.8 [kN]	19,3	30,7	44,7	84,0	130,7	188,0	244,7	299,3
	HIT-V-R [kN]	13,9	21,9	31,6	58,8	92,0	132,1	80,4	98,3
	HIT-V-HCR [kN]	19,3	30,7	44,7	84,0	130,7	117,6	152,9	187,1

### Design combined pull-out and concrete cone resistance

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Typical embedment depth $h_{ef} = h_{ef,typ}$ [mm]	80	90	110	125	170	210	240	270
Non-cracked concrete								
$N_{Rd,p}^0$ Temperature range I [kN]	22,3	31,4	46,1	69,8	118,7	175,9	169,6	212,1
$N_{Rd,p}^0$ Temperature range II [kN]	19,0	26,7	39,2	59,3	100,9	149,5	135,7	169,6
$N_{Rd,p}^0$ Temperature range III [kN]	15,6	22,0	32,3	48,9	83,1	123,2	124,4	155,5
Cracked concrete								
$N_{Rd,p}^0$ Temperature range I [kN]	6,7	9,4	18,4	27,9	47,5	70,4	90,5	113,1
$N_{Rd,p}^0$ Temperature range II [kN]	5,0	7,1	15,0	22,7	38,6	57,2	73,5	91,9
$N_{Rd,p}^0$ Temperature range III [kN]	4,5	6,3	12,7	19,2	32,6	48,4	62,2	77,8

$$\text{Design concrete cone resistance } N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$$

$$\text{Design splitting resistance } N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$$

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
$N_{Rd,c}^0$ Non-cracked concrete [kN]	20,1	24,0	32,4	39,2	62,2	85,4	104,3	124,5
$N_{Rd,c}^0$ Cracked concrete [kN]	14,3	17,1	23,1	28,0	44,3	60,9	74,4	88,7

a) Splitting resistance must only be considered for non-cracked concrete.

### Influencing factors

#### Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} =$	1,00	1,00	1,00	1,00	1,00	1,00	1,00

#### Influence of embedment depth on combined pull-out and concrete cone resistance

$f_{h,p} = h_{ef}/h_{ef,typ}$
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#### Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{0,5}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a)  $f_{ck,cube}$  = concrete compressive strength, measured on cubes with 150 mm side length

#### Influence of edge distance a)

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$										

a) The edge distance shall not be smaller than the minimum edge distance  $c_{min}$ . These influencing factors must be considered for every edge distance smaller than the critical edge distance.

### Influence of anchor spacing <sup>a)</sup>

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$s/s_{cr,sp}$										
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$										
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1

a) The anchor spacing shall not be smaller than the minimum anchor spacing  $s_{min}$ . This influencing factor must be considered for every anchor spacing.

### Influence of embedment depth on concrete cone resistance

$$f_{h,N} = (h_{ef}/h_{ef,typ})^{1,5}$$

### Influence of reinforcement

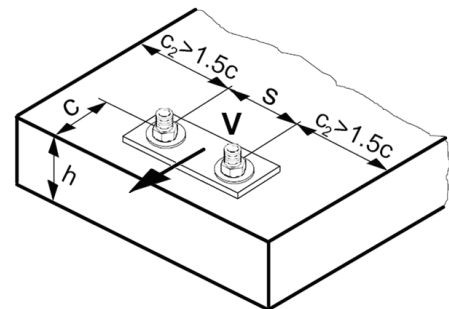
$h_{ef}$ [mm]	60	70	80	90	$\geq 100$
$f_{re,N} = 0,5 + h_{ef}/200mm \leq 1$	0,8 <sup>a)</sup>	0,85 <sup>a)</sup>	0,9 <sup>a)</sup>	0,95 <sup>a)</sup>	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing  $\geq 150$  mm (any diameter) or with a diameter  $\leq 10$  mm and a spacing  $\geq 100$  mm, then a factor  $f_{re,N} = 1$  may be applied.

## SHEAR loading

### The design shear resistance is the lower value of

- Steel resistance:  $V_{Rd,s}$
- Concrete pryout resistance:  $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance:  $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



### Basic design shear resistance

#### Design steel resistance $V_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
$V_{Rd,s}$	HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
	HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
	HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
	HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3

#### Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a)$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$$k = 2$$

a)  $N_{Rd,p}$ : Design combined pull-out and concrete cone resistance,  $N_{Rd,c}$ : Design concrete cone resistance

#### Design concrete edge resistance $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Non-cracked concrete									
$V_{Rd,c}^0$ [kN]		5,9	8,6	11,6	18,7	27,0	36,6	44,5	53,0
Cracked concrete									
$V_{Rd,c}^0$ [kN]		4,2	6,1	8,2	13,2	19,2	25,9	31,5	37,5

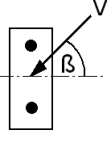
### Influencing factors

#### Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a)  $f_{ck,cube}$  = concrete compressive strength, measured on cubes with 150 mm side length

#### Influence of angle between load applied and the direction perpendicular to the free edge

Angle $\beta$	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_\beta = \sqrt{\frac{1}{(\cos \alpha_V)^2 + \left(\frac{\sin \alpha_V}{2,5}\right)^2}}$ 	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

#### Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

#### Influence of anchor spacing and edge distance <sup>a)</sup> for concrete edge resistance: $f_4$

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

c/h <sub>ef</sub>	Single anchor	Group of two anchors s/h <sub>ef</sub>														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing  $s_{min}$  and the minimum edge distance  $c_{min}$ .

### Influence of embedment depth

$h_{ef}/d$	4	4,5	5	6	7	8	9	10	11
$f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$	0,51	0,63	0,75	1,01	1,31	1,64	2,00	2,39	2,81
$h_{ef}/d$	12	13	14	15	16	17	18	19	20
$f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$	3,25	3,72	4,21	4,73	5,27	5,84	6,42	7,04	7,67

### Influence of edge distance <sup>a)</sup>

$c/d$	4	6	8	10	15	20	30	40
$f_c = (d / c)^{0,19}$	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance  $c_{min}$ .

## Combined TENSION and SHEAR loading

For combined tension and shear loading see section "Anchor Design".

### Precalculated values – design resistance values

All data applies to:

- non-cracked concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$
- temperature range I (see service temperature range)
- minimum thickness of base material
- no effects of dense reinforcement

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action  $\gamma = 1,4$ . The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

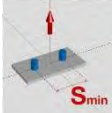
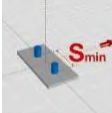
### Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - minimum embedment depth

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = h_{ef,min}$ [mm]	60	60	70	80	90	96	108	120
Base material thickness $h = h_{min}$ [mm]	90	90	100	116	138	152	168	190
<b>Tensile <math>N_{Rd}</math>: single anchor, no edge effects</b>								
Non-cracked concrete								
HIT-V 5.8 [kN]	12,0	13,0	16,4	20,1	24,0	26,4	31,5	36,9
HIT-V 8.8 [kN]	13,0	13,0	16,4	20,1	24,0	26,4	31,5	36,9
HIT-V-R [kN]	13,0	13,0	16,4	20,1	24,0	26,4	31,5	36,9
HIT-V-HCR [kN]	13,0	13,0	16,4	20,1	24,0	26,4	31,5	36,9
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	5,0	6,3	11,7	14,3	17,1	18,8	22,4	26,3
HIT-V-R / -HCR [kN]	5,0	6,3	11,7	14,3	17,1	18,8	22,4	26,3
<b>Shear <math>V_{Rd}</math>: single anchor, no edge effects, without lever arm</b>								
Non-cracked concrete								
HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	63,3	75,6	88,5
HIT-V 8.8 [kN]	12,0	18,4	27,2	48,2	57,5	63,3	75,6	88,5
HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	63,3	48,3	58,8
HIT-V-HCR [kN]	12,0	18,4	27,2	48,2	57,5	63,3	75,6	88,5
Cracked concrete								
HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	41,0	45,1	53,9	63,1
HIT-V 8.8 [kN]	12,0	15,1	27,2	34,3	41,0	45,1	53,9	63,1
HIT-V-R [kN]	8,3	12,8	19,2	34,3	41,0	45,1	48,3	58,8
HIT-V-HCR [kN]	12,0	15,1	27,2	34,3	41,0	45,1	53,9	63,1

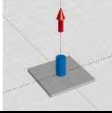
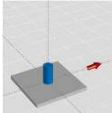
### Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - minimum embedment depth

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = h_{ef,min}$ [mm]	60	60	70	80	90	96	108	120
Base material thickness $h = h_{min}$ [mm]	90	90	100	116	134	152	168	190
Edge distance $c = c_{min}$ [mm]	40	50	60	80	100	120	135	150
<b>Tensile <math>N_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>)</b>								
Non-cracked concrete								
HIT-V 5.8 / 8.8 [kN]	7,1	7,8	9,7	12,8	16,5	20,7	24,2	28,9
HIT-V-R / -HCR [kN]	7,1	7,8	9,7	12,8	16,5	20,7	24,2	28,9
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	3,0	4,2	8,0	10,7	13,7	16,4	19,5	22,9
HIT-V-R / -HCR [kN]	3,0	4,2	8,0	10,7	13,7	16,4	19,5	22,9
<b>Shear <math>V_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>), without lever arm</b>								
Non-cracked concrete								
HIT-V 5.8 / 8.8 [kN]	3,5	4,9	6,6	10,2	13,9	17,9	21,5	25,9
HIT-V-R / -HCR [kN]	3,5	4,9	6,6	10,2	13,9	17,9	21,5	25,9
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	2,5	3,5	4,7	7,2	9,9	12,7	15,3	18,3
HIT-V-R / -HCR [kN]	2,5	3,5	4,7	7,2	9,9	12,7	15,3	18,3


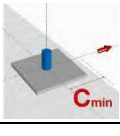
Design resistance: concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$  - minimum embedment depth  
(load values are valid for single anchor)

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = h_{ef,min}$ [mm]		60	60	70	80	90	96	108	120
Base material thickness $h = h_{min}$ [mm]		90	90	100	116	134	152	168	190
Spacing $s = s_{min}$ [mm]		40	50	60	80	100	120	135	150
	<b>Tensile <math>N_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>)</b>								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	7,7	7,9	10,0	12,6	15,4	17,9	21,2	25,0
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	3,5	4,4	7,5	9,5	11,7	13,3	15,9	18,6
		<b>Shear <math>V_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>), without lever arm</b>							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	39,4	44,9	53,5	62,7
HIT-V 8.8 [kN]		12,0	18,4	25,4	32,1	39,4	44,9	53,5	62,7
HIT-V-R [kN]		8,3	12,8	19,2	32,1	39,4	44,9	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	25,4	32,1	39,4	44,9	53,5	62,7
Cracked concrete									
HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]		7,2	9,6	16,8	22,9	28,1	32,0	38,2	44,7

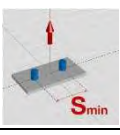
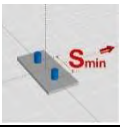
Design resistance: concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$  - typical embedment depth

Anchor size		M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]		80	90	110	125	170	210	240	270
Base material thickness $h = h_{min}$ [mm]		110	120	140	161	214	266	300	340
	<b>Tensile <math>N_{Rd}</math>: single anchor, no edge effects</b>								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	12,0	19,3	28,0	39,2	62,2	85,4	104,3	124,5
	HIT-V 8.8 [kN]	19,3	24,0	32,4	39,2	62,2	85,4	104,3	124,5
	HIT-V-R [kN]	13,9	21,9	31,6	39,2	62,2	85,4	80,4	98,3
	HIT-V-HCR [kN]	19,3	24,0	32,4	39,2	62,2	85,4	104,3	124,5
	<b>Shear <math>V_{Rd}</math>: single anchor, no edge effects, without lever arm</b>								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
	HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
	HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
	HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3
	Cracked concrete								
	HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2	
HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8	
HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3	

### Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - typical embedment depth

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]	80	90	110	125	170	210	240	270	
Base material thickness $h = h_{min}$ [mm]	110	120	140	161	214	266	300	340	
Edge distance $c = c_{min}$ [mm]	40	50	60	80	100	120	135	150	
	<b>Tensile <math>N_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>)</b>								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	9,6	11,6	15,5	19,9	30,5	41,5	50,5	60,0
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	3,6	5,2	10,2	16,5	25,2	34,2	41,5	49,3
		<b>Shear <math>V_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>), without lever arm</b>							
Non-cracked concrete									
HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]		3,7	5,3	7,3	11,5	17,2	23,6	29,0	34,8
Cracked concrete									
HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]		2,6	3,8	5,2	8,1	12,2	16,7	20,5	24,7

### Design resistance: concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$ - typical embedment depth (load values are valid for single anchor)

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = h_{ef,typ}$ [mm]	80	90	110	125	170	210	240	270	
Base material thickness $h = h_{min}$ [mm]	110	120	140	161	214	266	300	340	
Spacing $s$ [mm]	40	50	60	80	100	120	135	150	
	<b>Tensile <math>N_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>)</b>								
	Non-cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	11,2	13,5	18,1	22,4	35,1	48,1	58,6	69,9
	Cracked concrete								
	HIT-V 5.8 / 8.8 HIT-V-R / -HCR [kN]	4,6	6,4	11,6	17,0	26,5	36,2	44,2	52,6
		<b>Shear <math>V_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>), without lever arm</b>							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		12,0	18,4	27,2	50,4	78,4	112,8	147,2	177,0
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3
Cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		9,4	13,4	26,1	40,7	63,6	86,9	106,0	126,2
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]	9,4	13,4	26,1	40,7	63,6	70,9	92,0	110,3	



Design resistance: concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$  - embedment depth = 12 d<sup>a)</sup>

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = 12 d^a)$ [mm]	96	120	144	192	240	288	324	360
Base material thickness $h = h_{min}$ [mm]	126	150	174	228	284	344	384	430
<b>Tensile <math>N_{Rd}</math>: single anchor, no edge effects</b>								
Non-cracked concrete								
HIT-V 5.8 [kN]	12,0	19,3	28,0	52,7	82,0	118,0	153,3	187,3
HIT-V 8.8 [kN]	19,3	30,7	44,7	74,6	104,3	137,1	163,6	191,6
HIT-V-R [kN]	13,9	21,9	31,6	58,8	92,0	132,1	80,4	98,3
HIT-V-HCR [kN]	19,3	30,7	44,7	74,6	104,3	117,6	152,9	187,1
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	8,0	12,6	24,1	42,9	67,0	96,5	116,6	136,6
HIT-V-R / -HCR [kN]	8,0	12,6	24,1	42,9	67,0	96,5	116,6	136,6
<b>Shear <math>V_{Rd}</math>: single anchor, no edge effects, without lever arm</b>								
Non-cracked concrete								
HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3
Cracked concrete								
HIT-V 5.8 [kN]	7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]	12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
HIT-V-R [kN]	8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]	12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3

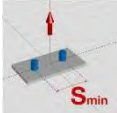
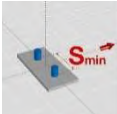
a) d = element diameter

Design resistance: concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$  - embedment depth = 12 d<sup>a)</sup>

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30
Embedment depth $h_{ef} = 12 d^a)$ [mm]	96	120	144	192	240	288	324	360
Base material thickness $h = h_{min}$ [mm]	126	150	174	228	284	344	384	430
Edge distance $c = c_{min}$ [mm]	40	50	60	80	100	120	135	150
<b>Tensile <math>N_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>)</b>								
Non-cracked concrete								
HIT-V 5.8 [kN]	11,8	16,5	21,7	33,4	46,7	61,3	73,2	85,7
HIT-V 8.8 [kN]	11,8	16,5	21,7	33,4	46,7	61,3	73,2	85,7
HIT-V-R [kN]	11,8	16,5	21,7	33,4	46,7	61,3	73,2	85,7
HIT-V-HCR [kN]	11,8	16,5	21,7	33,4	46,7	61,3	73,2	85,7
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	4,2	6,5	12,5	22,2	34,7	48,9	58,4	68,4
HIT-V-R / -HCR [kN]	4,2	6,5	12,5	22,2	34,7	48,9	58,4	68,4
<b>Shear <math>V_{Rd}</math>: single anchor, min. edge distance (<math>c = c_{min}</math>), without lever arm</b>								
Non-cracked concrete								
HIT-V 5.8 / 8.8 [kN]	3,9	5,7	7,8	12,9	18,9	25,9	31,8	38,1
HIT-V-R / -HCR [kN]	3,9	5,7	7,8	12,9	18,9	25,9	31,8	38,1
Cracked concrete								
HIT-V 5.8 / 8.8 [kN]	2,8	4,0	5,5	9,1	13,4	18,4	22,5	27,0
HIT-V-R / -HCR [kN]	2,8	4,0	5,5	9,1	13,4	18,4	22,5	27,0

a) d = element diameter

Design resistance: concrete C 20/25 –  $f_{ck,cube} = 25 \text{ N/mm}^2$  - embedment depth =  $12 d^a$   
(load values are valid for single anchor)

Anchor size	M8	M10	M12	M16	M20	M24	M27	M30	
Embedment depth $h_{ef} = 12 d^a$ [mm]	96	120	144	192	240	288	324	360	
Base material thickness $h = h_{min}$ [mm]	126	150	174	228	284	344	384	430	
Spacing $s = s_{min}$ [mm]	40	50	60	80	100	120	135	150	
	<b>Tensile <math>N_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>)</b>								
	Non-cracked concrete								
	HIT-V 5.8 [kN]	12,0	19,3	26,5	40,8	57,0	74,9	89,4	104,6
	HIT-V 8.8 [kN]	14,4	20,1	26,5	40,8	57,0	74,9	89,4	104,6
	HIT-V-R [kN]	13,9	20,1	26,5	40,8	57,0	74,9	80,4	98,3
	HIT-V-HCR [kN]	14,4	20,1	26,5	40,8	57,0	74,9	89,4	104,6
	Cracked concrete								
	HIT-V 5.8 / 8.8 [kN]	5,5	8,5	15,4	26,5	40,1	55,7	66,4	77,8
	HIT-V-R / -HCR [kN]								
		<b>Shear <math>V_{Rd}</math>: double anchor, no edge effects, min. spacing (<math>s = s_{min}</math>), without lever arm</b>							
Non-cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		12,0	18,4	27,2	50,4	78,4	112,8	147,2	179,2
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]		12,0	18,4	27,2	50,4	78,4	70,9	92,0	110,3
Cracked concrete									
HIT-V 5.8 [kN]		7,2	12,0	16,8	31,2	48,8	70,4	92,0	112,0
HIT-V 8.8 [kN]		11,0	17,2	27,2	50,4	78,4	112,8	147,2	179,2
HIT-V-R [kN]		8,3	12,8	19,2	35,3	55,1	79,5	48,3	58,8
HIT-V-HCR [kN]	11,0	17,2	27,2	50,4	78,4	70,9	92,0	110,3	

a)  $d$  = element diameter