



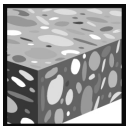


Hilti HIT-HY 200 with HIS-(R)N

Injection mortar system		Benefits
	Hilti HIT-HY 200-A 500 ml foil pack (also available as 330 ml)	<ul style="list-style-type: none"> - Suitable for cracked and non-cracked concrete C 20/25 to C 50/60. - Suitable for dry and water saturated concrete - High loading capacity, excellent handling, and fast curing - Small edge distance and anchor spacing possible - Corrosion resistant - In service temperature range up to 120°C short term/72°C long term - Manual cleaning for anchor size M8 and M10 - Two mortar (A and R) versions available with different curing times and same performance
	Hilti HIT-HY 200-R 500 ml foil pack (also available as 330 ml)	
	Static mixer	
	Internal threaded sleeve HIS-N HIS-RN	



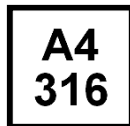
Concrete



Tensile zone



Small edge distance and spacing



Corrosion resistance



European Technical Approval



CE conformity



PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval ^{a)}	DIBt, Berlin	ETA-11/0493 / 2012-08-08 (Hilti HIT-HY 200-A) ETA-12/0084 / 2012-08-08 (Hilti HIT-HY 200-R)

a) All data given in this section according ETA-11/0493 and ETA-12/0084, issue 2012-08-08.

Basic loading data (for a single anchor)

All data in this section applies to

For details see Simplified design method

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Steel failure
- Base material thickness, as specified in the table
- One anchor material, as specified in the tables
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$
- Temperate range I
(min. base material temperature -40°C , max. long term/short term base material temperature: $+24^\circ\text{C}/40^\circ\text{C}$)
- Installation temperature range -10°C to $+40^\circ\text{C}$

Embedment depth and base material thickness for the basic loading data. Mean ultimate resistance, characteristic resistance, design resistance, recommended loads.

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth	h_{ef} [mm]	90	110	125	170	205
Base material thickness	h [mm]	120	150	170	230	270

Mean ultimate resistance: concrete C 20/25 , anchor HIS-N with screw 8.8

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Non cracked concrete						
Tensile $N_{Ru,m}$	HIS-N [kN]	26,3	48,3	70,4	123,9	114,5
Shear $V_{Ru,m}$	HIS-N [kN]	13,7	24,2	41,0	62,0	57,8
Cracked concrete						
Tensile $N_{Ru,m}$	HIS-N [kN]	26,3	48,3	66,8	105,9	114,5
Shear $V_{Ru,m}$	HIS-N [kN]	13,7	24,2	41,0	62,0	57,8

Characteristic resistance: concrete C 20/25 , anchor HIS-N with screw 8.8

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Non cracked concrete						
Tensile N_{Rk}	HIS-N [kN]	25,0	46,0	67,0	111,9	109,0
Shear V_{Rk}	HIS-N [kN]	13,0	23,0	39,0	59,0	55,0
Cracked concrete						
Tensile $N_{Ru,m}$	HIS-N [kN]	24,7	39,9	50,3	79,8	105,7
Shear $V_{Ru,m}$	HIS-N [kN]	13,0	23,0	39,0	59,0	55,0

Design resistance: concrete C 20/25 , anchor HIS-N with screw 8.8

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Cracked concrete						
Tensile N_{Rd}	HIS-N [kN]	17,5	30,7	44,7	74,6	74,1
Shear V_{Rd}	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
Non cracked concrete						
Tensile $N_{Ru,m}$	HIS-N [kN]	16,5	26,6	33,5	53,2	70,4
Shear $V_{Ru,m}$	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7

Recommended loads ^{a)}: concrete C 20/25 , anchor HIS-N with screw 8.8

Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205
Non cracked concrete							
Tensile N_{rec}	HIS-N	[kN]	12,5	27,9	31,9	53,3	53,0
Shear V_{rec}	HIS-N	[kN]	7,4	13,1	18,6	28,1	26,2
Cracked concrete							
Tensile $N_{Ru,m}$	HIS-N	[kN]	11,8	19,0	24,0	38,0	50,3
Shear $V_{Ru,m}$	HIS-N	[kN]	7,4	13,1	18,6	28,1	26,2

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Service temperature range

Hilti HIT-HY 200 injection mortar may be applied in the temperature ranges given below. An elevated base material temperature may lead to a reduction of the design bond resistance.

Temperature range	Base material temperature	Maximum long term base material temperature	Maximum short term base material temperature
Temperature range I	-40 °C to +40 °C	+24 °C	+40 °C
Temperature range II	-40 °C to +80 °C	+50 °C	+80 °C
Temperature range III	-40 °C to +120 °C	+72 °C	+120 °C

Max short term base material temperature

Short-term elevated base material temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

Max long term base material temperature

Long-term elevated base material temperatures are roughly constant over significant periods of time.

Materials

Mechanical properties of HIS-(R)N

Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205
Nominal tensile strength f_{uk}	HIS-N	[N/mm ²]	490	460	460	460	460
	Screw 8.8	[N/mm ²]	800	800	800	800	800
	HIS-RN	[N/mm ²]	700	700	700	700	700
	Screw A4-70	[N/mm ²]	700	700	700	700	700
Yield strength f_{yk}	HIS-N	[N/mm ²]	410	375	375	375	375
	Screw 8.8	[N/mm ²]	640	640	640	640	640
	HIS-RN	[N/mm ²]	350	350	350	350	350
	Screw A4-70	[N/mm ²]	450	450	450	450	450
Stressed cross-section A_s	HIS-(R)N	[mm ²]	51,5	108,0	169,1	256,1	237,6
	Screw	[mm ²]	36,6	58	84,3	157	245
Moment of resistance W	HIS-(R)N	[mm ³]	145	430	840	1595	1543
	Screw	[mm ³]	31,2	62,3	109	277	541

Material quality

Part	Material
Internal threaded sleeve ^{a)} HIS-N	C-steel 1.0718, Steel galvanized $\geq 5\mu\text{m}$
Internal threaded sleeve ^{b)} HIS-RN	Stainless steel 1.4401 and 1.4571

a) related fastening screw: strength class 8.8, A5 > 8% Ductile
steel galvanized $\geq 5\mu\text{m}$

b) related fastening screw: strength class 70, A5 > 8% Ductile
stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362

Anchor dimensions

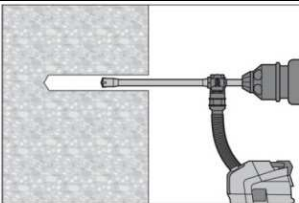
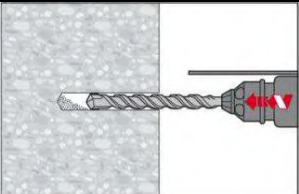
Anchor size	M8x90	M10x110	M12x125	M16x170	M20x205
Internal threaded sleeve HIS-N / HIS-RN					
Embedment depth h_{ef} [mm]	90	110	125	170	205

Setting

installation equipment

Anchor size	M8x90	M10x110	M12x125	M16x170	M20x205
Rotary hammer	TE 2 – TE 16		TE 40 – TE 70		
Other tools	compressed air gun or blow out pump, set of cleaning brushes, dispenser				

Setting instruction

Bore hole drilling	
	Drill hole to the required embedment depth with an appropriately sized Hilti TE-CD or TE-YD hollow drill bit with Hilti vacuum attachment. This drilling method properly cleans the borehole and removes dust while drilling. After drilling is complete, proceed to the "injection preparation" step in the instructions for use.
	Drill Hole to the required embedment depth with a hammer drill set in rotation-hammer mode using an appropriately sized carbide drill bit.

Bore hole cleaning Just before setting an anchor, the bore hole must be free of dust and debris.

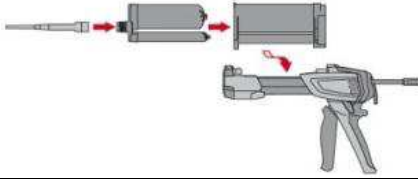
a) Manual Cleaning (MC) non-cracked concrete only
for bore hole diameters $d_0 \leq 20\text{mm}$ and bore hole depth $h_0 \leq 10d$

	<p>The Hilti manual pump may be used for blowing out bore holes up to diameters $d_0 \leq 20\text{ mm}$ and embedment depths up to $h_{ef} \leq 10d$. Blow out at least 4 times from the back of the bore hole until return air stream is free of noticeable dust</p>
	<p>Brush 4 times with the specified brush size by inserting the steel brush Hilti HIT-RB to the back of the hole (if needed with extension) in a twisting motion and removing it. The brush must produce natural resistance as it enters the bore hole -- if not the brush is too small and must be replaced with the proper brush diameter.</p>
	<p>Blow out again with manual pump at least 4 times until return air stream is free of noticeable dust.</p>

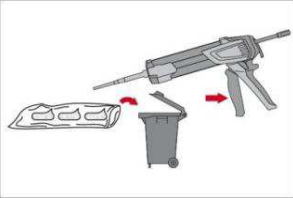
b) Compressed air cleaning (CAC)
for all bore hole diameters d_0 and all bore hole depth h_0

	<p>Blow 2 times from the back of the hole (if needed with nozzle extension) over the hole length with oil-free compressed air (min. 6 bar at 6 m³/h) until return air stream is free of noticeable dust.</p>
	<p>Brush 2 times with the specified brush size by inserting the steel brush Hilti HIT-RB to the back of the hole (if needed with extension) in a twisting motion and removing it. The brush must produce natural resistance as it enters the bore hole -- if not the brush is too small and must be replaced with the proper brush diameter.</p>
	<p>Blow again with compressed air 2 times until return air stream is free of noticeable dust.</p>

Injection preparation



Tightly attach new Hilti mixing nozzle HIT-RE-M to foil pack manifold (snug fit). Do not modify the mixing nozzle. Observe the instruction for use of the dispenser. Check foil pack holder for proper function. Do not use damaged foil packs / holders. Swing foil pack holder with foil pack into HIT-dispenser.



Discard initial adhesive. The foil pack opens automatically as dispensing is initiated. Depending on the size of the foil pack an initial amount of adhesive has to be discarded.

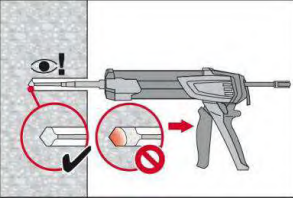
Discard quantities are

2 strokes for 330 ml foil pack,

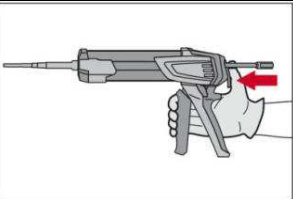
3 strokes for 500 ml foil pack,

4 strokes for 500 ml foil pack $\leq 5^{\circ}\text{C}$.

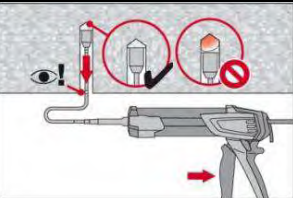
Inject adhesive from the back of the borehole without forming air voids



Inject the adhesive starting at the back of the hole, slowly withdrawing the mixer with each trigger pull. Fill holes approximately 2/3 full, or as required to ensure that the annular gap between the anchor and the concrete is completely filled with adhesive along the embedment length.



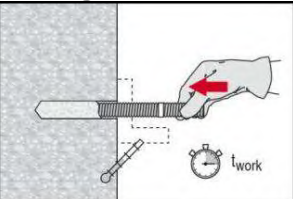
After injection is completed, depressurize the dispenser by pressing the release trigger. This will prevent further adhesive discharge from the mixer.



Overhead installation.

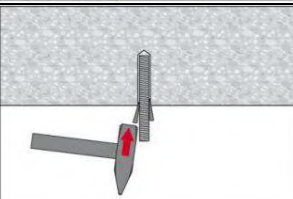
For overhead installation the injection is only possible with the aid of extensions and piston plugs. Assemble HIT-RE-M mixer, extension(s) and appropriately sized piston plug. Insert piston plug to back of the hole and inject adhesive. During injection the piston plug will be naturally extruded out of the bore hole by the adhesive pressure.

Setting the element

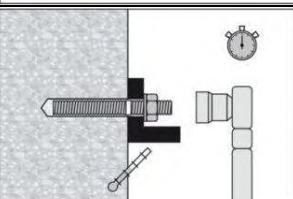


Before use, verify that the element is dry and free of oil and other contaminants.

Mark and set element to the required embedment depth until working time t_{work} has elapsed.



For overhead installation use piston plugs and fix embedded parts with e.g. wedges



Loading the anchor:

After required curing time t_{cure} the anchor can be loaded.

The applied installation torque shall not exceed T_{max} .

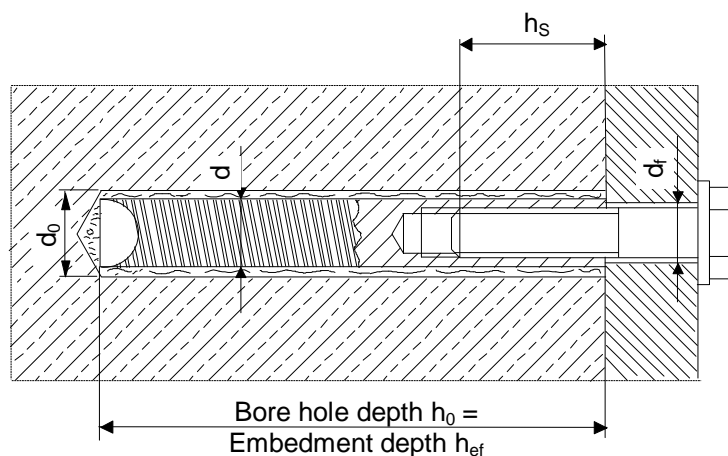
For detailed information on installation see instruction for use given with the package of the product.

Working time, curing time

Temperature of the base material	Hilti HIT-HY 200-R	
	Working time in which anchor can be inserted and adjusted t_{work}	Curing time before anchor can be fully loaded t_{cure}
-10 °C to -5 °C	3 hour	20 hour
-4 °C to 0 °C	2 hour	7 hour
1 °C to 5 °C	1 hour	3 hour
6 °C to 10 °C	40 min	2 hour
11 °C to 20 °C	15 min	1 hour
21 °C to 30 °C	9 min	1 hour
31 °C to 40 °C	6 min	1 hour

Temperature of the base material	Hilti HIT-HY 200-A	
	Working time in which anchor can be inserted and adjusted t_{work}	Curing time before anchor can be fully loaded t_{cure}
-10 °C to -5 °C	1,5 hour	7 hour
-4 °C to 0 °C	50 min	4 hour
1 °C to 5 °C	25 min	2 hour
6 °C to 10 °C	15 min	1 hour
11 °C to 20 °C	7 min	30 min
21 °C to 30 °C	4 min	30 min
31 °C to 40 °C	3 min	30 min

Setting details



Anchor size			M8x90	M10x110	M12x125	M16x170	M20x205	
Nominal diameter of drill bit	d_0	[mm]	14	18	22	28	32	
Diameter of element	d	[mm]	12,5	16,5	20,5	25,4	27,6	
Effective anchorage and drill hole depth	h_{ef}	[mm]	90	110	125	170	205	
Minimum base material thickness	h_{min}	[mm]	120	150	170	230	270	
Diameter of clearance hole in the fixture	d_f	[mm]	9	12	14	18	22	
Thread engagement length; min - max	h_s	[mm]	8-20	10-25	12-30	16-40	20-50	
Torque moment ^{a)}	T_{max}	[Nm]	10	20	40	80	150	
Minimum spacing	s_{min}	[mm]	40	45	55	65	90	
Minimum edge distance	c_{min}	[mm]	40	45	55	65	90	
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	$2 c_{cr,sp}$					
Critical edge distance for splitting failure ^{b)}	$c_{cr,sp}$	[mm]	$1,0 \cdot h_{ef}$ for $h / h_{ef} \geq 2,0$					
			$4,6 h_{ef} - 1,8 h$ for $2,0 > h / h_{ef} > 1,3$					
			$2,26 h_{ef}$ for $h / h_{ef} \leq 1,3$					
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	$2 c_{cr,N}$					
Critical edge distance for concrete cone failure ^{c)}	$c_{cr,N}$	[mm]	$1,5 h_{ef}$					

For spacing (or edge distance) smaller than critical spacing (or critical edge distance) the design loads have to be reduced.

- a) Maximum recommended torque moment to avoid splitting failure during installation with minimum spacing and/or edge distance.
- b) h : base material thickness ($h \geq h_{min}$), h_{ef} : embedment depth
- c) The critical edge distance for concrete cone failure depends on the embedment depth h_{ef} and the design bond resistance. The simplified formula given in this table is on the safe side.

Simplified design method

Simplified version of the design method according ETAG 001, TR 029. Design resistance according data given in ETA-11/0493 issued 2012-08-08 for HIT-HY 200-A and ETA-12/0084 issued 2012-08-08 for HIT-HY 200-R. Both mortars possess identical technical load performance.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge distance. The influencing factors must then be considered for each edge distance and spacing. The simplified calculated design loads take a conservative approach: They will be lower than the exact values according to ETAG 001, TR 029. For an optimized design, anchor calculation can be performed using PROFIS anchor design software.

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

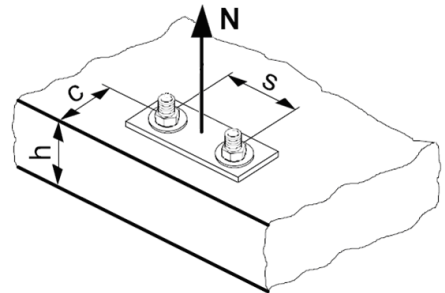
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

TENSION loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Combined pull-out and concrete cone resistance:
$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete):
$$N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
$N_{Rd,s}$	HIS-N with screw 8.8 [kN]	17,5	30,7	44,7	80,3	74,1
	HIS-RN with screw A4-70 [kN]	13,9	21,9	31,6	58,8	69,2

Design combined pull-out and concrete cone resistance

$$N_{Rd,p} = N_{Rd,p}^0 \cdot f_{B,p} \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,p} \cdot f_{re,N}$$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth	h_{ef} [mm]	90	110	125	170	205
Non cracked concrete						
$N_{Rd,p}^0$	Temperature range I [kN]	30,6	49,4	69,8	117,6	154,7
$N_{Rd,p}^0$	Temperature range II [kN]	25,9	41,8	59,0	99,5	130,4
$N_{Rd,p}^0$	Temperature range III [kN]	22,4	36,1	51,0	85,9	112,6
Cracked concrete						
$N_{Rd,p}^0$	Temperature range I [kN]	16,5	26,6	37,6	63,3	83,0
$N_{Rd,p}^0$	Temperature range II [kN]	13,0	20,9	29,5	49,7	65,2
$N_{Rd,p}^0$	Temperature range III [kN]	11,8	19,0	26,8	45,2	59,3

$$\text{Design concrete cone resistance } N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{h,N} \cdot f_{re,N}$$

$$\text{Design splitting resistance } N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,N} \cdot f_{re,N}$$

Anchor size		M8	M10	M12	M16	M20
Non cracked concrete						
$N_{Rd,c}^0$	[kN]	28,7	38,8	47,1	74,6	98,8
Cracked concrete						
$N_{Rd,c}^0$	[kN]	20,5	27,7	33,5	53,2	70,4

a) Splitting resistance must only be considered for non-cracked concrete.

Influencing factors

Influence of concrete strength on combined pull-out and concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_{B,p} = (f_{ck,cube}/25N/mm^2)^{0,10}$ a)	$f_{B,p} = 1$						

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of embedment depth on combined pull-out and concrete cone resistance

$f_{h,p} = 1$

Influence of concrete strength on concrete cone resistance

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{0,5}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance a)

$c/c_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$c/c_{cr,sp}$										
$f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$										
$f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1
$f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$										

a) The edge distance shall not be smaller than the minimum edge distance c_{min} . These influencing factors must be considered for every edge distance smaller than the critical edge distance.

Influence of anchor spacing ^{a)}

$s/s_{cr,N}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$s/s_{cr,sp}$										
$f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$										
$f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} . This influencing factor must be considered for every anchor spacing.

Influence of embedment depth on concrete cone resistance

$f_{h,N} = 1$

Influence of reinforcement

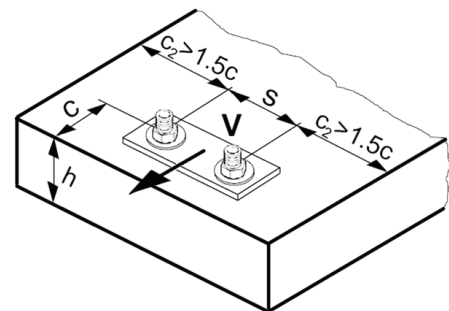
h_{ef} [mm]	40	50	60	70	80	90	≥ 100
$f_{re,N} = 0,5 + h_{ef}/200mm \leq 1$	0,7 ^{a)}	0,75 ^{a)}	0,8 ^{a)}	0,85 ^{a)}	0,9 ^{a)}	0,95 ^{a)}	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N} = 1$ may be applied.

SHEAR loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot \text{lower value of } N_{Rd,p} \text{ and } N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
$V_{Rd,s}$	HIS-N with screw 8.8 [kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN with screw A4-70 [kN]	8,3	12,8	19,2	35,3	41,5

Design concrete pryout resistance $V_{Rd,cp} = \text{lower value}^a)$ of $k \cdot N_{Rd,p}$ and $k \cdot N_{Rd,c}$

$k = 2$

- a) $N_{Rd,p}$: Design combined pull-out and concrete cone resistance
 $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_\beta \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

Anchor size		M8	M10	M12	M16	M20
Non-cracked concrete						
$V_{Rd,c}^0$ [kN]		12,4	19,6	28,2	40,2	46,2
Cracked concrete						
$V_{Rd,c}^0$ [kN]		8,8	13,9	20,0	28,5	32,7

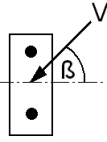
Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25\text{N/mm}^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle β	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_\beta = \frac{1}{\sqrt{(\cos \alpha_v)^2 + \left(\frac{\sin \alpha_v}{2,5}\right)^2}}$ 	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4
 $f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$

c/h _{ef}	Single anchor	Group of two anchors s/h _{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

Influence of embedment depth

Anchor size	M8	M10	M12	M16	M20
$f_{hef} =$	1,38	1,21	1,04	1,22	1,45

Influence of edge distance ^{a)}

c/d	4	6	8	10	15	20	30	40
$f_c = (d / c)^{0,19}$	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance c_{min} .

Combined TENSION and SHEAR loading

For combined tension and shear loading see section "Anchor Design".

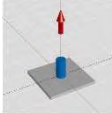
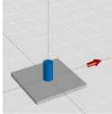
Precalculated values – design resistance values

All data applies to:


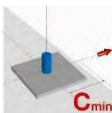
- non-cracked concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$
- temperature range I (see service temperature range)
- minimum thickness of base material
- no effects of dense reinforcement

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

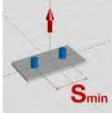
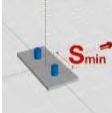
Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth h_{ef} [mm]		90	110	125	170	205
Base material thickness $h = h_{min}$ [mm]		120	150	170	230	270
	Tensile N_{Rd}: single anchor, no edge effects					
	Non-cracked concrete					
	HIS-N [kN]	17,5	30,7	44,7	74,6	74,1
	HIS-RN [kN]	13,9	21,9	31,6	58,8	69,2
	Cracked concrete					
	HIS-N [kN]	16,5	26,6	33,5	53,2	70,4
HIS-RN [kN]	13,9	21,9	31,6	53,2	69,2	
	Shear V_{Rd}: single anchor, no edge effects, without lever arm					
	Non-cracked concrete					
	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN [kN]	8,3	12,8	19,2	35,3	41,5
	Cracked concrete					
	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
HIS-RN [kN]	8,3	12,8	19,2	35,3	41,5	

Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth h_{ef} [mm]		90	110	125	170	205
Base material thickness $h = h_{min}$ [mm]		120	150	170	230	270
Edge distance $c = c_{min}$ [mm]		40	45	55	65	90
	Tensile N_{Rd}: single anchor, min. edge distance ($c = c_{min}$)					
	Non-cracked concrete					
	HIS-N [kN]	13,1	17,5	21,6	33,1	44,9
	HIS-RN [kN]	13,1	17,5	21,6	33,1	44,9
	Cracked concrete					
	HIS-N [kN]	8,4	13,2	17,1	25,9	35,9
HIS-RN [kN]	8,4	13,2	17,1	25,9	35,9	
	Shear V_{Rd}: single anchor, min. edge distance ($c = c_{min}$), without lever arm					
	Non-cracked concrete					
	HIS-N [kN]	4,2	5,5	7,6	10,8	17,2
	HIS-RN [kN]	4,2	5,5	7,6	10,8	17,2
	Cracked concrete					
	HIS-N [kN]	3,0	3,9	5,4	7,7	12,2
HIS-RN [kN]	3,0	3,9	5,4	7,7	12,2	

Design resistance: non-cracked- concrete C 20/25

Anchor size		M8x90	M10x110	M12x125	M16x170	M20x205
Embedment depth	h_{ef} [mm]	90	110	125	170	205
Base material thickness	$h = h_{min}$ [mm]	120	150	170	230	270
Spacing	$s = s_{min}$ [mm]	40	45	55	65	90
Tensile N_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$)						
Non-cracked concrete						
	HIS-N [kN]	15,8	21,3	25,9	40,6	54,3
	HIS-RN [kN]	13,9	21,3	25,9	40,6	54,3
Cracked concrete						
	HIS-N [kN]	10,1	15,4	19,2	30,0	40,4
	HIS-RN [kN]	10,1	15,4	19,2	30,0	40,4
Shear V_{Rd}: double anchor, no edge effects, min. spacing ($s = s_{min}$), without lever arm						
Non-cracked concrete						
	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN [kN]	8,3	12,8	19,2	35,3	41,5
Cracked concrete						
	HIS-N [kN]	10,4	18,4	26,0	39,3	36,7
	HIS-RN [kN]	8,3	12,8	19,2	35,3	41,5

